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# Analysis of a Cable Stayed Suspension Hybrid Bridge Considering Vehicular Loading Using CSI

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#### ARTICLEINFO

ABSTRACT

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Page Number 73-83 Cable-stayed bridges are built for providing connectivity over obstacles such as rivers, mountains, flyovers over rotary squares and valleys for a long span. Cable stayed bridge provides ample stability and utilises structure material and further its advantages goes in for cheap maintenance and design. Design of the bridge is highly dependent on its function and purpose and the nature of terrain of the site. A bridge type known as a suspension bridge has its deck suspended by vertical suspenders and suspension cables. The main structural elements of a suspension bridge system are stiffening girders/trusses, main suspension cables, main towers, and cable anchorages at each end of the bridge. Vertical suspenders sustain the weight of the deck and the traffic load, while the main cables are stretched between towers and eventually connect to the anchorage or the bridge itself. The superstructures of suspension bridges are constructed utilising the cable erection technique similarly to other cable-supported bridges.

The primary load-bearing parts are the main cables, which are tension members made of high-strength steel. Buckling is not a concern because the main cable's full cross-section is very effective at moving weights. As a result, the deadweight of the bridge can be greatly decreased, enabling a longer span. Suspension bridges also look better than other types of bridges when compared to their appearance.

The ease of both a suspension bridge and a cable-stayed bridge is offered by a hybrid bridge. The main objective of this research is to examine the behaviour of three cases—a cable-stayed bridge, a suspension bridge, and a cable-stayed suspension hybrid bridge—under similar loading situations. CSI Bridge's analytical programme is used to model and analyse the three situations while taking seismic loading and vehicle loads into account.

**Keywords :** Cable Stayed Bridge (CSB); Suspension Bridge (SB); Cable Stayed Suspension Hybrid Bridge (CSSHB),Pylon, Seismic Analysis ,CSI Bridge.

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#### I. INTRODUCTION

Long span bridge are increasing day by day to facilitate the need of construction projects. Cable stayed bridges and suspension bridges are the systems used to achieve long span bridges. The spans of cablesupported bridges range from 200 to 2000 metres. The greatest span of a cable-supported bridge system is determined by the density, stiffness, and strength of the materials utilised. In the design and analysis of cable-stayed bridges and suspension bridges, high strength steel cables are a key component, which is superior for effective tension resistance. The advantages of a cable-supported bridge are in the way that the system uses the materials.

A suspension bridge is a form of bridge in which the deck is suspended on vertical suspenders from below suspension cables. In the early 1800s, the first examples of this kind of bridge were constructed in the modern era. In many hilly areas of the world, simple suspension bridges with no vertical suspenders have a long history.

Thousand years ago, people cross water bodies with the help of cable attached with wooden block. It was born of cable stayed and suspension bridge but mainly cable stayed bridge developed in 1595 and commonly used in 19th century. In early days, Cable Bridge was constructed with combination of suspension bridge and cable stayed bridge. In 1808 an American inventor named James give the born of modern cable bridge. Two cables are used over the top of many towers and anchoring this chain on the either side of bridge structure. Although suspension bridges and cable stayed bridges are quite similar, the main distinction between the two is how the deck force is transferred to the cable in a suspension bridge. In a cable stayed bridge, the cable is connected directly to the deck. Because of its low weight, improved aesthetics, and long span designs, cable stayed bridges have gained popularity. The primary purpose of a cable-stayed bridge is to handle its own weight and impending traffic loads safely while also being aesthetically beautiful and demonstrating excellent serviceability under any conceivable load scenario. Steel wire cable is used to suspend a cable stayed bridge deck. The top vertical towers are fixed with these cables, which transmit shear force to the vertical members, which then transform it into compression force.

In this study, three distinct cases—a suspension bridge, a cable-stayed bridge, and a hybrid cablestayed suspension bridge—are largely examined. CSI Bridge's analytical application was used to model the data and conduct the analysis while accounting for seismic and vehicle loading.

## Suspension Bridge

In a suspension bridge a deck is supported by vertical hangers, which are connected to main catenary cables. These primary catenary cables are cradled by pylons and secured at an anchor point at the end of the span. Modern suspension bridges can cross greater lengths than any other type of bridge because they are lightweight, aesthetically beautiful, and strong. They are also some of the most expensive to build. Although suspension bridges can be built to be sturdy enough to accommodate freight trains, they were almost all created with vehicular traffic in mind.

Vertical loads are carried by tensioned, curved cables in suspension bridges. These loads are passed to the anchorages, which must withstand the inward and occasionally vertical pull of the cables, as well as the towers, which carry them by vertical compression to the ground. The towers are the only parts of the suspension bridge that are in compression, making it appear as an upside-down arch under tension. The deck must be carefully controlled to prevent excessive movement because it is suspended in the air. Therefore, the deck needs to be either heavy, stiff, or both.

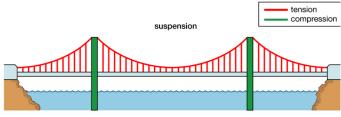


Fig 1 Suspension Bridge

#### Construction

Caissons are sunk into the riverbed and filled with concrete to create foundations for bridges spanning bodies of water that need piers. Large wooden, metal, or concrete boxes or cylinders are called caissons. For suspension bridges, the caissons are covered with towers. Originally made of stone, suspension-bridge towers are now made of steel or concrete. The anchorages are then constructed on both ends, often out of reinforced concrete with embedded steel eyebars for attaching the cables. A piece of metal with an opening (or "eye") at each end is called an eyebar. However, cables are often composed of thousands of steel wires spun together at the construction site, as opposed to the early suspension bridges that used cables formed of linked wrought-iron eyebars. Rope pulleys used for spinning transport each wire from one mooring to the other and back across the tops of the towers. To stop corrosion, the wires are then bundled and coated. The deck is built after the cables are finished, typically by floating deck sections out aboard ships, raising them with cranes, and fastening them to the suspenders.

#### History and Uses

Suspension bridges are among the earliest types of engineering, having been built by prehistoric peoples who used vines as cables and mounted the road or walkway directly on the wires. A far more durable form that hung the roadway using cables made of plaited bamboo and later iron chain was developed in India around the fourth century CE.

In the modern era, the suspension bridge offered a practical solution to the issue of long spans over navigable streams or at other locations where stream piers are hard to come by. In the late 18th and early 19th centuries, British, French, American, and other engineers experienced significant issues with stability and strength against wind forces and heavy loads; failures were caused by storms, huge snowfalls, and herds of cattle. The primary credit for resolving goes to John Augustus Roebling, a German-born American engineer who added a web truss to either side of his roadways and created a structure so rigid that he was able to span the Niagara Gorge in Niagara Falls, New York, the Ohio River in Cincinnati, and, finally, his masterpiece, the Brooklyn Bridge in New York City, which spans the East River between Brooklyn and Manhattan.

## II. Literature Survey

**Rajni Verma and Rashmi Sakalle (2022)** in the research paper, girder Bridge and cable stayed bridge was modelled and comparative analysis was carried out for dynamically loading conditions. A comparison was made between bridges for dead load, live load and combined load. Structural analysis was done to determine internal forces, stresses and deformation of structure under various load effects. the modelling and analysis of both the cases was performed using analytical application SAP 2000.

The support response in the Cable stayed bridge is 1091.65 K.N., whereas the Girder bridge is 1427.87 K.N., indicating that stresses are properly distributed in the Cable stayed bridge scenario. As a result, in the case of a cable-stayed bridge, the support reaction is 21.11 percent lower. Based on the results of the moment, forces, and deflection given in the preceding chapter, it can be concluded that a Cable Stayed bridge is more stable in resisting load. When comparing the cost of a cable-stayed bridge to the cost of a girder bridge, the cable-stayed bridge is 18% more expensive. The results showed that the cable-stayed bridge was more stable and adequate for big loads, while the girder bridge was more cost effective.



#### Objectives of the research

The main objective of the research is to study the behaviour of cable-stayed bridge, suspension bridge and Cable-Stayed Suspension Hybrid Bridge. To accomplish the objectives, following studies are analyzed-

- Seismic behaviour of cable stayed bridge, suspension bridge and hybrid bridge
- To determine the effectiveness of hybrid bridge in comparison to suspenson and cable stayed bridge.
- To determine the stability of all the thee cases considering vehicular loading as per I.R.C.
- To determine the utilization of Csi bridge software considering Indan Codal Provision.

#### III.METHODOLOGY

**Step 1** research papers were analyzed from different authors who have investigated behaviour of cable stayed bridge and suspension bridge on different loading conditions. Different tools were used for understanding the behaviour of the models and the values were validated on ground of axial load, omen and bending moment using SAP2000 and Staad.Pro, while some authors calculated the values manually.

**Step 2** Designing the model of all the three cases using CsiBridge where the materials, tendons, bent design, section data, bridge restrainer data, foundation spring data, bridge bearing data and bridge abutment data is defined and analyzed.

Step 3 Defining material properties for three bridges where the material properties were predefined via Csi bridge application for concrete, steel and tendons.

Material Name and Display Color		M30		
Material Type		Concret	е	$\sim$
Material Grade		M30		
Material Notes		М	odify/S	Show Notes
Weight and Mass				Units
Weight per Unit Volume	24.9926			KN, m, C $\sim$
Mass per Unit Volume	2.5485			
Isotropic Property Data				
Modulus Of Elasticity, E				27386128.
Poisson, U				0.2
Coefficient Of Thermal Expansio	n, A			1.300E-05
Shear Modulus, G				11410887.
Other Properties For Concrete Ma	iterials			
Specified Concrete Compressive	e Strength	, fc		30000.
Expected Concrete Compressive	e Strength			30000.
Lightweight Concrete				
Shear Strength Reduction F	actor			
] Switch To Advanced Property [	Display			

#### Fig 2 Defining concrete property

General Data Material Name and Display Color	Fe345
Material Type	Steel 🗸
Material Grade	Fe345
Material Notes	Modify/Show Notes
Weight and Mass	Units
Weight per Unit Volume 76	9729 KN, m, C 🗸 🗸
Mass per Unit Volume 7.8	349
Isotropic Property Data	
Modulus Of Elasticity, E	2.100E+08
Poisson, U	0.3
Coefficient Of Thermal Expansion, A	A 1.170E-05
Shear Modulus, G	80769231.
Other Properties For Steel Materials	
Minimum Yield Stress, Fy	345000.
Minimum Tensile Stress, Fu	450000.
Expected Yield Stress, Fye	379500.
Expected Tensile Stress, Fue	495000.
Switch To Advanced Property Disp	lay
ОК	Cancel

#### Fig 3 Defining Steel Property

Step 4 Defining Bridge Bent data as cap beam length of 9.15m with number of columns as 1 and single bearing line is considered for continuous superstructure.

Bridge Bent Name	Units	Girder Support Condition	
BENT1	KN, m, C	<ul> <li>Integral</li> </ul>	
		<ul> <li>Connect to Girder Bottom Only</li> </ul>	
Bent Data			
Cap Beam Section +	FSEC1	~	
Cap Beam Length	9.15		
		-	
Number of Columns	1		
Modify/Show	Column Data		
Bent Type			
Single Bearing Line (Continuou	s Superstructure)		
O Double Bearing Line (Discontin	uous Superstructure)		

Fig 4 Bridge Bent Design

**Step 5** Defining Bridge Bearing data. A bridge bearing is a component of a bridge which typically provides a resting surface between bridge piers and the bridge deck. The purpose of a bearing is to allow controlled movement and thereby reduce the stresses involved. Here in this step a customized name is given to bridge bearing along with its defining units and consider support property for link.

Bridge Bearing Name	BBRG1		KN, m, C
Bridge Bearing Is Defined By: Link/Support Property User Definition		+ LINK1	~

#### Fig 5 Bridge Bearing Data

**Step 6** In order to maintain the embankment and transfer the vertical and horizontal stresses from the superstructure to the foundation, abutments are employed at the ends of bridges. Here the abutment is connected to the girder along with the foundation spring substructure.

Bridge Abutment Name	BABT1		Units KN, m, C	~
Girder Support Condition				
O Integral				
Connect to Girder Bottom Or	nly			
Substructure Type				
Foundation Spring				
O Continuous Beam (Continuo	usly Supported)			
Section Property		+		
Beam Length				
Foundation Spring				
Foundation Spring Property		+ Fixed		$\sim$
Note: When substructure type line spring.	is grade beam, fo	undation spring pro	perty represents a	

## Fig 6 Bridge Abutment Data

**Step 7** Defining Foundation Spring Data which is user defined as fixed release type

oundation Spring Name	21	Units KN, m, C
undation Spring Is Defined By:		
) Link/Support Property	+	
User Definition		
Property is Defined for This Length	in a Line Spring	1.
Property is Defined for This Length Property is Defined for This Area in		1.
Property is Defined for This Length Property is Defined for This Area in ser Foundation Spring	an Area Spring	1.
Property is Defined for This Length Property is Defined for This Area in er Foundation Spring DOF/Direction		1.
Property is Defined for This Length Property is Defined for This Area in ser Foundation Spring	an Area Spring Release Ty	1.
Property is Defined for This Length Property is Defined for This Area in the Foundation Spring DOF/Direction Translation Vertical (U1)	an Area Spring Release Ty Fixed	1.
Property is Defined for This Length Property is Defined for This Area in the Foundation Spring DOF/Direction Translation Vertical (U1) Translation Along Skew (U2)	an Area Spring           Release Ty           Fixed	1.
Property is Defined for This Length Property is Defined for This Area in the Foundation Spring DOF/Direction Translation Vertical (U1) Translation Along Skew (U2) Translation Normal to Skew (U3)	an Area Spring           Release Ty           Fixed           Fixed	1.

#### Fig 7 Foundation Spring Data

**Step 8** Defining Bridge Restrainer Data. Restrainers are used to reduce deck unseating over sub structures due to earthquakes. The main effect of restraints upon global bridge motions is found to constrain and redistribute the relative distances between adjacent vibrations units. The bridge modeller enables modelling of cable bridges. Here the restrainer length and area is defined.



Bridge Restrainer Name BRST1	KN, m, C
Bridge Restrainer Type	
C Link/Support Property	+
User Definition	
User Restrainer	
Restrainer Length	0.6096
Restrainer Area	6.452E-04
Restrainer Modulus E	1.999E+08
Restrainer Slack Length	0.

Fig 8 Bridge Restrainer Data

**Step 9** Defining lane data for deck.

	ne Name	LANE1	
Frame	Centerline O	ffset Lane Wid	ith
1	1.	0	Add
			Insert
			Modify
			Delete
Reverse O	rder Reverse Si	ign Move La	ne
ane Edge Type	e	Maximum La	ne Load Discretization Lengths
ane Edge Type Left Edge	e Interior ~	Maximum La Along Lane	
Left Edge			3.
Left Edge Right Edge	Interior $\checkmark$	Along Lane Across Lar	3. 3.
Left Edge Right Edge dditional Lane	Interior $\checkmark$ Interior $\checkmark$	Along Lane Across Lar	3. 3.
Left Edge Right Edge dditional Lane Discretizati	Interior ~ Interior ~ Load Discretization Pa	Along Lane Across Lar maneters Along Lan Than 1/	a. 3. 3. 3. e

## Fig 9 Lane Data

**Step 10** Defining section properties for tendons and here the tendons size was similar in all the three cases.

Tendon Section Name	TEN1
Section Notes	Modify/Show
Tendon Modeling Options Fo	or Analysis Model
O Model Tendon as Load	ds
Model Tendon as Elem	nents
Tendon Parameters	
Prestress Type	Prestress ~
Material Property	+ Tendon $\checkmark$
Tendon Properties	
O Specify Tendon Diame	6.452E-04
Specify Tendon Area	6.452E-04
Torsional Constant	6.625E-08
Moment of Inertia	3.312E-08
Shear Area	5.806E-04
Units KN, m, C V	Display Color

Fig 10 Tendons section Data

**Step 11** Defining vehicular data for vehicular loading condition

/ehicle Name	Design Type		Units		
VEH-1	Vehicle Live	~	KN, m, C 🔍		
Source			Notes		
Source: User Defined	Conve	rt to User Defined	Notes		
ength Effects				Load Plan	
Axie	AASHTO impact	~	Modify/Show		
Uniform	AASHTO Impact	~	Modify/Show		
<ul> <li>Vehicle Applies To Strat</li> <li>Straddle Reduction Fa</li> <li>Vehicle Remains Fully In</li> </ul>				Load Elevation Modify/Show Loads Vertical Loading	Horizontal Loading
		Nin Dist Allowed			for contraction contraction
Jsage Lane Negative Moments	at Consula			Center of Gravity	0.
Interior Vertical Support		Lane Exterior Er	-	Height - Axle Loads	
All other Responses		Lane Interior Ed	ge 0.	Height - Uniform Lo	ede 0.

## Fig 11 Vehicle Data

**Step 12** Defining and assigning loading condition along with vehicular loading condition as per IRC 6 and seismic load as per IS 1893:2016.

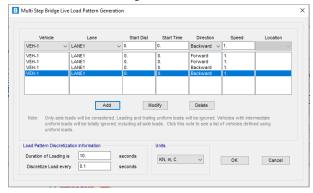


Fig 12 Multi Step Bridge Live Load Pattern Generation



Г

Load Direction and Diaphragm Eccentricity	Seismic Coefficients
Global X Direction	Seismic Zone Factor, Z
O Global Y Direction	Per Code
Ecc. Ratio (All Diaph.) 0.05	O User Defined
	Soil Type 🛛 🗸 🗸
Override Diaph. Eccen. Override	Importance Factor, I 1.
Time Period	Factors
O Approximate Ct (m) =	Response Reduction, R 5.
Program Calc	
O User Defined ⊤ =	
Lateral Load Elevation Range	
Program Calculated	
O User Specified Reset Defaults	OK

# Fig 13 Seismic Load Pattern

**Step 13** Analyzing the structure for all the models for deformation

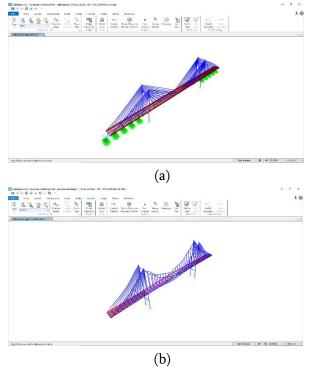


Fig 3.14 Analyzing the structure for deformity Table 1 Description of Structure

Geometrical Des	scription of Cable Stayed Bridge
Main Span	300m
Left Span	150m
Right Span	150m
Width of deck	30m

Depth of Deck	3m
Type of Deck	Beam Slab
Deck Level	15m
Pattern of Cable	Harp
Height of Pylon	35m
Height of Left Pylon	25m
Height of Right Pylon	20m
Type of Pier	Solid Rectangular
Type of Foundation	Footing
Pier in Left Span	5
Pier of Right Span	5
Type of Pylon	H Frame
Height above Deck	100m
Type of Foundation	Fixed
Total Depth	40m

# Table 2 Geometrical Description of Suspension Bridge

Geometrical Description of Suspension Bridge		
Length of Left Span	20m	
Length of Middle Span	80m	
Length of Right Span	20m	
Width of Deck	3m	
Height of Column H1	5m	
Number of Divisions		
N1	6	
Number of Divisions		
N2	24	



Number of Divisions N3	6
Minimum Middle Sag	2
Height of Column H2	10

## Table 3 Geometrical Description of Hybrid Bridge

Geometrical Description of Hybrid Cable Stayed Suspension Bridge		
Suspens	aon bridge	
Main Span	300m	
Left Span	150m	
Right Span	150m	
Width of deck	30m	
Depth of Deck	3m	
Type of Deck	Beam Slab	
Deck Level	15m	
Pattern of Cable at the end	Harp	
Pattern of cable in center	Straight	
Height of Pylon	35m	
Height of Left Pylon	25m	
Height of Right Pylon	20m	
Type of Pier	Solid Rectangular	
Type of Foundation	Footing	
Pier in Left Span	5	
Pier of Right Span	5	
Type of Pylon	H Frame	
Height above Deck	100m	
Type of Foundation	Fixed	
Total Depth	40m	

## IV. RESULTS AND DISCUSSION

Shear Force in kN		
Suspension Cable Stayed Hybrid Bridge		

Bridge	Bridge	
890.394	799.207	791.007

Discussion: When two fastened structures (or two components of a single structure) are pulled apart, shear stress results. The shear stress can figuratively rip bridge materials in half if unchecked. Here the shear force was least in case of Hybrid bridge as 791.007 kN whereas maximum shear force was visible in Suspension bridge as 890.394kN. The shear force was 5% less in hybrid bridge in comparison to Cable stayed bridge and 11 % less in comparison to Suspension Bridge.

Displacement in mm		
Suspension	Cable Stayed	
Bridge	Bridge	Hybrid Bridge
621.098	598.786	593.909

Discussion: Displacement is the separation of one node or element (such as a pier, deck, pylon, or set of tendons) from its original position. The movement may be caused by a beam deflecting, but it may also be the consequence of the complete object moving, undistorted, as a box sliding across a friction-free surface. The maximum deflection in this example was 621.098 mm for the suspension bridge, 598.768 mm for the cable-stayed bridge, and 593.909 mm for the hybrid bridge. It should be mentioned that suspension bridges operate better when their length is greater than 400 metres.

Support Reaction in kN		
Suspension		
Bridge	Cable Stayed Bridge	Hybrid Bridge
4432.932	5132.09	5210.032

Discussion: A support response might be a force acting on a support or an end moment that follows from a movement that is not possible. Support reactions and external forces operating on structural systems are in a state of equilibrium. There are six



support responses applied from the fixed support to the structure since the joint with fixed support has no degrees of freedom. Here, the hybrid bridge saw the highest level of support reaction, which is 6% higher than a cable-stayed bridge and 9% more than a suspension bridge.

Maximum Moment in kN-m		
Suspension Cable Stayed		
Bridge	Bridge	Hybrid Bridge
1111.88	1098.27	998.213

Discussion: The maximum instant occurs when the shear force is zero or changes sign (positive to negative or vice-versa). The Suspension bridge showed the greatest moment, but the Hybrid bridge showed the least.

# V. CONCLUSION

This research is primarily focused towards analyzing three different cases namely suspension bridge, cable stayed bridge and Hybrid cable stayed suspension bridge. The modelling and analysis was performed using analytical application CSI bridge considering vehicular loading and seismic loading.

# Shear Force

When two fastened structures (or two components of a single structure) are pulled apart, shear stress results. The shear stress can figuratively rip bridge materials in half if unchecked. In this scenario, the shear force was lowest for the hybrid bridge at 791.007 kN and highest for the suspension bridge at 890.394 kN. In comparison to a cable-stayed bridge and a suspension bridge, the shear force in a hybrid bridge was 5 and 11 percent lower, respectively.

## Displacement

Displacement is the separation of one node or element (such as a pier, deck, pylon, or set of tendons) from its original position. The movement may be caused by a beam deflecting, but it may also be the consequence of the complete object moving, undistorted, as a box sliding across a friction-free surface. The maximum deflection in this example was 621.098 mm for the suspension bridge, 598.768 mm for the cable-stayed bridge, and 593.909 mm for the hybrid bridge. It should be mentioned that suspension bridges operate better when their length is greater than 400 metres.

## Torsion

Torsion is the state of strain in a material that has been twisted by an applied torque. It will occur whenever a structural element is subject to a twisting force. Torsion was maximum in suspension bridge in comparison to other two cases. For instance, engineers who construct suspension bridges must pay close attention to torsion. The suspended roadway rotates and twists like a rolling wave as a result of strong wind.

## Support Reaction

A support response is a force acting on a support or an end moment that follows from a movement that is not possible. Support reactions and external forces operating on structural systems are in a state of equilibrium. There are six support responses applied from the fixed support to the structure since the joint with fixed support has no degrees of freedom. Here, the hybrid bridge had the highest support reaction—6 percent higher than a cable-stayed bridge and 9 percent higher than a suspension bridge.

## Maximum Moment

The maximum moment is maximum where shear force is zero or its changes sign (positive to negative or vice-versa). Maximum moment was visible in Suspension bridge whereas the least was visible in Hybrid bridge.

## VI. FUTURE SCOPE

• The hybrid bridge can be designed with different span with increasing the range of suspended deck and cable stayed deck in order to identify its behaviour in case of seismic load.



- The modelling and analysis can be done using different analytical application SAP 2000 and Staad.pro.
- Consider live truck loads run gon the deck with live vehicular loading.
- Perform collapse analysis and wind analysis on similar models to identify its behaviour on application of such loads.

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