

# Comparative Analysis of Filler Wall Interlock System Using Three Different Wall Material Using ETABS

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# ABSTRACT

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## Article History

Accepted : 01 June 2022 Published : 10 June 2022 A reinforced concrete building with masonry infill is most common type of construction in India. Traditionally, conventional clay bricks or concrete blocks which are heavy rigid materials have been used as Infill wall. Though, AAC (aerated light weight concrete) blocks which are lightweight, flexible building materials that provides insulation and fire resistance and have lower impact on environment, can be used as masonry infill (MI) material in buildings. AAC blocks are now also available in India. A number of researchers have studied the behavior of AAC in-filled reinforced concrete (R/C) frames experimentally. Interlocking Bricks are even introduced further which adds an advantage over AAC bricks.

Hence in the report, comparative study of the effect of type of infill wall material on wind response of structure has been presented. AAC blocks, Interlocking Blocks and conventional clay bricks materials are used for the comparison. To check the behavior of RC frames with interlocking blocks, AAC block and conventional clay bricks infill, analysis has been done using ETABS. Three models are considered for comparison. One is infill with conventional bricks, 2nd is infill with AAC blocks and third is infill frame with open ground storey. To model the infill analytically, equivalent diagonal strut method is used. The ends of diagonal strut are pin jointed. Infill behaves like compression strut between column and beam and compression forces are transferred from one node to another. The analysis has been carried out for dead load (DL) live load (LL), and wind load. The results have shown that AAC block infill and interlocking blocks material behaves better under lateral loading than conventional brick. The study of the effect of types of infill materials used on the performance of RC infilled frames is still limited.

Index Terms: Interlocking Blocks, AAC block, Infill frame, Equivalent strut model.



## I. INTRODUCTION

Reinforced concrete building with masonry infill is the most common type of construction in India. Masonry walls are provided for functional and architectural point of view and thus they are generally considered as nonstructural elements. Hence interaction of infill with bonding frame is neglected in the design. Though an infill panel interacts with the bonding frame and may induce a load resistance mechanism when subjected to lateral loads. Influence of infill is ignored in modeling of the RC structure which leads to inaccuracy in guessing the actual seismic behavior of framed structures. Infilled frame shows a composite structure which is made by the combination of both RC frame and Infill walls. The Infill walls in infilled frame may be of conventional clay brick, concrete block or AAC block. The study of the influence of types of infill materials on the seismic response of infilled RC frames is still limited.

Thus, in present study focus is given on the effect of type of material on wind performance. AAC blocks, Interlocking Blocks and clay bricks are used as infill in RC frame. AAC blocks are light-weight building materials that provide insulation and fire resistance and have lower impact on environment.

Behaviors of in-filled RC frames have been studied by number researchers of experimentally and analytically. Conclusion is made by them that infill materials influence the lateral response of the in-filled frame significantly. Infill materials improve the performance of RC frame structure. An infill wall decreases lateral deflections, storey drift and bending moments in the frame and increases axial forces in the column thus reduce the probability of collapse. Hence, considering the infill leads to slender frame members in design, reducing the overall cost of the structural system.

## 1.2 Masonry Infill

The infill wall is the supported wall that closes the perimeter of a building constructed with a threedimensional framework structure (generally made of steel or reinforced concrete). Therefore, the structural frame ensures the bearing function, whereas the infill wall serves to separate inner and outer space, filling up the boxes of the outer frames. The infill wall has the unique static function to bear its own weight. The infill wall is an external vertical opaque type of closure. With respect to other categories of wall, the infill wall differs from the partition that serves to separate two interior spaces, yet also non-load bearing, and from the load bearing wall. The latter performs the same functions of the infill wall, hygro-thermally and acoustically, but performs static functions too. The use of masonry infill walls, and to some extent veneer walls, especially in reinforced concrete frame structures, is common in many countries. In fact, the use of masonry infill walls offers an economical and durable solution. They are easy to build, attractive for architecture and have a very efficient costperformance.

Today, masonry enclosures and partition walls are mainly made of clay units, but also aggregate concrete units (dense and lightweight aggregate) and autoclaved aerated concrete units are used. More recently, industry is also trying to introduce wood concrete blocks. Partition walls, made with both vertically and horizontally perforated clay blocks, represent two-thirds of the corresponding market.





Fig 1 Masonry Infill

# II. Objectives of the research

Following are the object of this study:

- To study the effects of infill materials on the behavior of RC frame under lateral loading.
- To evaluate the behavior of RC frames infilled with AAC blocks as the lightweight materials and clay brick simulating lateral forces and compare the results in terms of Displacement, Column forces, Beam forces, Storey shear, Base shear and Storey drift.
- To evaluate the behavior of RC frames infilled with interlocking blocks simulating lateral forces and compare the results in terms of Displacement, Column forces, Beam forces, Storey shear, Base shear and Storey drift.
- To find out green and environmentally safe materials such as AAC blocks which can be used in place of conventional bricks and perform better in seismic prone areas.
- To find out effect of infill masonry frame in reinforced concrete buildings compared to bare frame under extreme wind prone area.
- To compare the different parameters such as deflection, storey drift, storey shear, axial forces and base shear in bare frame and infill frame.

## **III. LITERATURE REVIEW**

Ashish Patil and Dr. Ajay G. Dhake (2021) research paper examined the layout notion of the interconnecting structure block and its application as a wall. The walls have their strength characteristics tested using ansys software. Similarly, the use of the particular interconnecting construction block below does not raise electricity more handy, but further minimises the amount of human effort. Such blocks may transfer from one location to another without any problem. The concrete grade M50 has been used for block instruction in the software application.

When comparing the RCC wall with the concrete wall triggered, the strain on the precast wall is significantly less than the RCC wall. The simplified building cycle reduces time, boosts productivity, reliability and efficiency and reduces costs. Precast Construction provides an extended lifetime and reduced maintenance costs. Precast concrete is more densely resistant to chemical attack, erosion, shock, ground suction and is dust-resistant.

Irfan Khan et.al (2021) research paper tested the computational modeling of Dry-Stack Block Masonry (DSM) walls subjected to cyclic monotonic loading. The analytical results were compared with experimental test results of the unreinforced and unconfined DSM cantilever walls subjected to lateral loading along with a constant axial load. ABAQUS has been used for Finite Element Modelling and analysis of the wall. Various material properties are defined for the wall in the software and modelled as a homogeneous material.

The results obtained from experimental work and numerical analysis using ABAQUS, of DSM walls showed a well-defined similarity in behaviour in the elastic range. The damage patterns of experimental and numerical model for monotonic lateral displacement are closely matching, which shows the authenticity of the Finite Element Package. The monotonic response of multi-story DSM can be



predicted accurately based on the results of the ABAQUS software. Though the pattern of Hysteresis loop, obtained from ABAQUS does not accurately match with the experimental plot, yet the cyclic

behaviour of DSM can be predicted. The plastic strains match with the corresponding experimental diagonal cracks for cyclic loading.

# IV. Steps of Modelling and Analysis

Step 1- the research papers from researchers were summarized at the beginning of the project in order to study the research done till date. Seismic analysis and wind analysis was conducted from different authors considered different types of infill system to understand the behaviour of the structure.

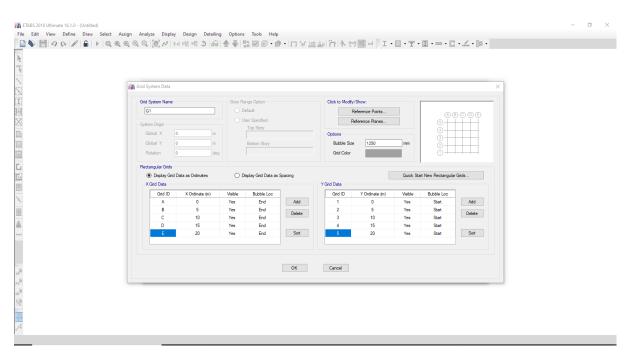
Step 2- ETABS support the option of quick template as per the grip system, here G+14 storey structure is considered with typical storey height is 3m and Bottom storey height us 3m.

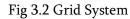
Grid Dimensions (Plan)		Story Dimensions		
O Uniform Grid Spacing		Simple Story Data		
Number of Grid Lines in X Direction		Number of Stories	14	
Number of Grid Lines in Y Direction		Typical Story Height	3	m
Spacing of Grids in X Direction		Bottom Story Height	3	m
Spacing of Grids in Y Direction				
Specify Grid Labeling Options	Grid Labels			
Oustom Grid Spacing		O Custom Story Data		
Specify Data for Grid Lines	Edit Grid Data	Specify Custom Story Data	Edit Story Data	
Add Structural Objects	HIII	Flat Slab Flat Slab with Perimeter Beams	Waffle Slab Two Way Ribbed St	

Fig 3.1 New Model Quick Templates



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Step 3- The frame is designed using ETABS in X and Y direction for G+14 storey structure where the height is defined in Z direction. The gap in the grid along the X and Y direction is 5m.

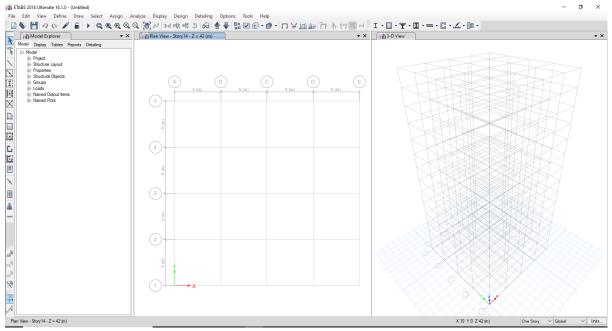


Fig 3.3 Design of the Frame

Step 4- Defining the properties of material for concrete, steel, slab and properties of infill. Here in this research, M30 concrete and HYSD 415 rebar is considered.

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General Data			
Material Name	M30		
Material Type	Concrete		$\sim$
Directional Symmetry Type	Isotropic		$\sim$
Material Display Color		Change	
Material Notes	Modi	fy/Show Notes	
Material Weight and Mass			
Specify Weight Density	O Spe	ecify Mass Density	
Weight per Unit Volume		24.9926	kN/m³
Mass per Unit Volume		2548.538	kg/m³
Mechanical Property Data			
Modulus of Elasticity, E		27386.13	MPa
Poisson's Ratio, U		0.2	
Coefficient of Thermal Expansion, A		0.000055	1/C
Shear Modulus, G		11410.89	MPa
Design Property Data			
Modify/Show M	laterial Propert	y Design Data	]
Advanced Material Property Data			
Nonlinear Material Data		Material Damping P	roperties
Time D	ependent Prop	perties	
ОК		Cancel	

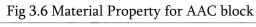
Fig 3.4 Material Property of Concrete

General Data			_
Material Name	HYSD415		
Material Type	Rebar		$\sim$
Directional Symmetry Type	Uniaxial		
Material Display Color		Change	
Material Notes	Modi	fy/Show Notes	
Material Weight and Mass			
Specify Weight Density	O Spe	cify Mass Density	
Weight per Unit Volume		76.9729	kN/m³
Mass per Unit Volume		7849.047	kg/m³
Mechanical Property Data			
Modulus of Elasticity, E		200000	MPa
Coefficient of Thermal Expansion, A		0.0000117	1/C
Design Property Data			
Modify/Show Ma	terial Property	y Design Data	
Advanced Material Property Data			
Nonlinear Material Data		Material Damping Pr	operties
Time De	pendent Prop	erties	



Material Property Data			
General Data			
Material Name	AAC BLOCK	4	7
Material Type		<b>u</b>	
	Masonry		~
Directional Symmetry Type	Isotropic		~
Material Display Color		Change	
Material Notes	Mod	fy/Show Notes	
Material Weight and Mass			
Specify Weight Density	O Spe	ecify Mass Density	
Weight per Unit Volume		21.2068	kN/m³
Mass per Unit Volume		2162.493	kg/m³
Mechanical Property Data			
Modulus of Elasticity, E		3600000	MPa
Poisson's Ratio, U		0.2	
Coefficient of Thermal Expansion, A		0.000081	1/C
Shear Modulus, G		1500000	MPa
Design Property Data			
Modify/Show Ma	aterial Propert	y Design Data	
Advanced Material Property Data			
Nonlinear Material Data		Material Damping Pro	perties
Time De	pendent Prop	perties	
ОК		Cancel	

Fig 3.5 Material Property of Rebar



General Data			
Material Name	Masonry		
Material Type	Masonry		$\sim$
Directional Symmetry Type	Isotropic		$\sim$
Material Display Color		Change	
Material Notes	Modif	y/Show Notes	
Material Weight and Mass			
Specify Weight Density	🔘 Spe	cify Mass Density	
Weight per Unit Volume		21.2068	kN/m³
Mass per Unit Volume		2162.493	kg/m³
Mechanical Property Data			
Modulus of Elasticity, E		3600000	MPa
Poisson's Ratio, U		0.2	
Coefficient of Thermal Expansion, A		0.000081	1/C
Shear Modulus, G		1500000	MPa
Design Property Data			
Modify/Show M	laterial Property	Design Data	
Advanced Material Property Data			
Nonlinear Material Data		Material Damping P	roperties
Time D	ependent Prop	erties	
	_	Cancel	

Fig 3.7 Property for Masonry Wall

Step	5-	Defi	ning	section	pro	perties	for	beam,	column,	slab	and	infill	wall.
	-	-					-	,	,				

Frame Section Property Data		>
General Data		
Property Name	BEAM	
Material	мзо 🗸	2 🏠
Notional Size Data	Modify/Show Notional Size	3
Display Color	Change	↓ ↓
Notes	Modify/Show Notes	
Shape		
Section Shape	Concrete Rectangular V	
Section Property Source Source: User Defined		Property Modifiers
Section Dimensions		Modify/Show Modifiers
Depth	400 mm	Currently Default
Width	200 mm	Reinforcement Modify/Show Rebar
		buuuuuuud
		ОК
Sh	ow Section Properties	Cancel

Fig 3.8 Section Property of Beam

General Data		
Property Name	COLUMN	
Material	M30 ~	2
Notional Size Data	Modify/Show Notional Size	• •
Display Color	Change	• <del>&lt; +</del> •
Notes	Modify/Show Notes	• •
Shape		• • •
Section Shape	Concrete Rectangular V	
Section Dimensions		Modify/Show Modifiers Currently Default
Section Dimensions		Modify/Show Modifiers
Depth	400 mm	rcement
Width	400 mm	Modify/Show Rebar
		OK

Fig 3.9 Section Property of Column



ieneral Data	
Property Name	AAC BLOCK
Property Type	Specified $\checkmark$
Wall Material	AAC BLOCK $\checkmark$
Notional Size Data	Modify/Show Notional Size
Modeling Type	Shell-Thin $\checkmark$
Modifiers (Currently Default)	Modify/Show
Display Color	Change
Property Notes	Modify/Show
roperty Data	
Thickness	200 mm

Fig 3.10 Data of Wall Property for AAC block

Property Name	Wall1	
Property Type	Specified	~
Wall Material	Masonry	×
Notional Size Data	Modify/Show Notional Size	
Modeling Type	Shell-Thin	$\sim$
Modifiers (Currently Default)	Modify/Show	
Display Color	Change	
Property Notes	Modify/Show	
Property Data		
Thickness	200	mm

Fig 3.11 Data of Wall Property for Masonry Wall

Step 6- Assigning Fixed Support at bottom of the structure for X, Y and Z direction

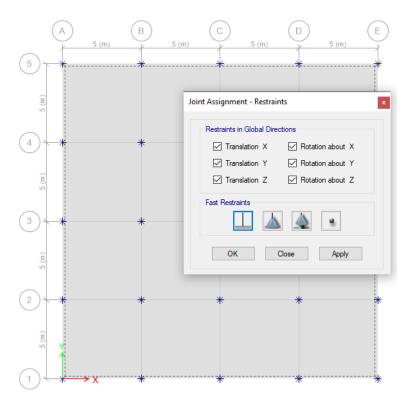
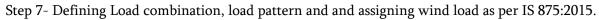


Fig 3.12 Defining Support Condition



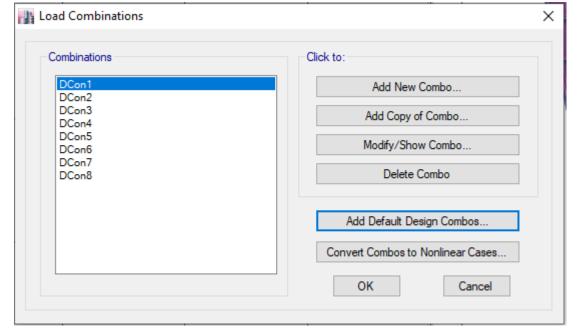


Fig 3.13 Load Combination



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fine Load Patterns					
oads		Self Weight	Auto	Click To:	
Load	Туре	Multiplier	Lateral Load	Ad	ld New Load
WIND	Wind	~ 0	Indian IS 875:2015	~ N	Nodify Load
Dead	Dead	1			
Live	Live	0		Modify	y Lateral Load
				D	elete Load
				ОК	Cancel

Fig 3.14 Defining Load Pattern

Exposure and Pressure Coefficients	Wind Coefficients	
Exposure from Extents of Diaphragms	Wind Speed, Vb (m/s)	39
Exposure from Shell Objects	Terrain Category	2 ~
	Importance Factor	1.15 ~
Vind Exposure Parameters	Risk Coefficient (k1 Factor)	1
Wind Directions and Exposure Widths	dify/Show	1
Windward Coefficient, Cp 0.8	Exposure Height	
Leeward Coefficient, Cp 0.5	Top Story	Story14 ~
	Bottom Story	Base 🗸 🗸
	Include Parapet	
	Parapet Height	<b>1.2</b> m

Fig 3.15 Wind Load as per IS 875:2015.

Step 8- Analyzing the structure on parameters of displacement, shear force, bending moment, axial force and base shear.

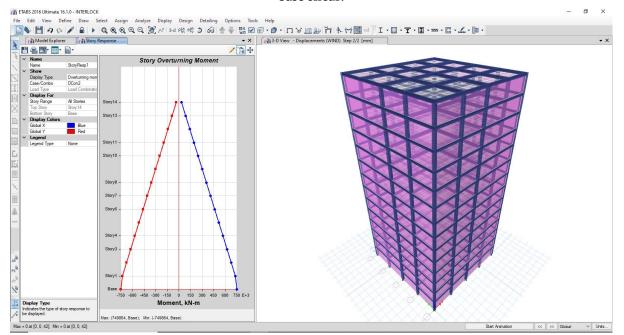


Fig 3.16 Overturning Moment



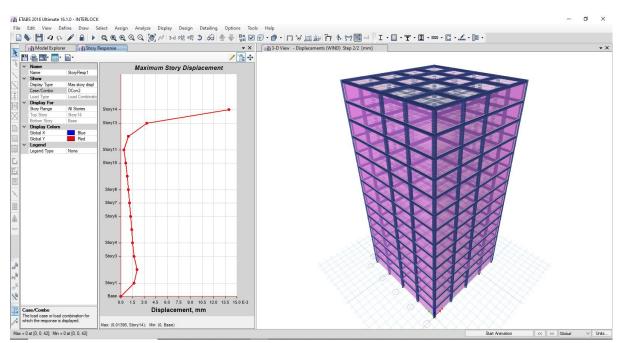


Fig 3.19 Storey Displacement

Step 9- The last step involves tabulation of results obtained from the analysis and presenting a comparative analysis of all the three cases considering the same loading conditions. Table 1 Geometrical Properties

Table 1 Geometrical Troperties			
Type of Structure	G+14		
Plan dimensions	$13.62 \text{ m} \times 16.30 \text{ m}$		
Height of each storey	3 m		
Foundation Level to ground	3 m		
Size of beams	400x200 mm		
Size of Column	400x400 mm		
thickness of slab	200 mm		
AAC wall Thickness	200 mm		
Masonry wall thickness	200 mm		
Interlocking Wall Thickness	200 mm		

## ANALYSIS RESULT:

Table 2 Storey Displacement in mm

Storey displacement in mm			
Floor	Masonry Infill	AAC Block Infill	Interlocking Block Infill
Storey 14	27	25	24
Storey 13	23	22	21



Storey 12	20	18	17
Storey 11	17	16	15
Storey 10	14	13	12
Storey 09	13	12	11
Storey 08	12	11	10
Storey 07	10	9	8
Storey 06	9	8	7
Storey 05	7	7	6
Storey 04	5	5	4
Storey 03	4	3	3
Storey 02	2	2	2
Storey 01	1	1	1
Base	1	1	1

# Table 3 Storey Drift in mm

	Storey drift in mm			
Floor	Masonry Infill	AAC Block Infill	Interlocking Block Infill	
Storey 14	0.000239	0.000222	0.000201	
Storey 13	0.000369	0.000336	0.000334	
Storey 12	0.00047	0.000425	0.000421	
Storey 11	0.000533	0.000483	0.000481	
Storey 10	0.000599	0.000514	0.000509	
Storey 09	0.000639	0.00061	0.00059	
Storey 08	0.000701	0.000689	0.000681	
Storey 07	0.000659	0.000623	0.00062	
Storey 06	0.00061	0.000583	0.00058	
Storey 05	0.000563	0.00052	0.00049	
Storey 04	0.000538	0.000505	0.000501	
Storey 03	0.000491	0.000474	0.000469	
Storey 02	0.000432	0.000433	0.000435	
Storey 01	0.0003	0.000308	0.000309	
Base	0.0003	0.000308	0.00031	



	Axial Forc	e in kN in column		
Floor	Masonry Infill	AAC Block Infill	Interlocking Block Infill	
Storey 14	352.23	266.99	164.101	
Storey 13	771.56	556.98	454.091	
Storey 12	1162.15	825.63	722.741	
Storey 11	1529.37	1077.3	974.411	
Storey 10	1875.09	1313.05	1210.161	
Storey 09	2200.56	1533.74	1430.851	
Storey 08	2505.93	1739.45	1636.561	
Storey 07	2790.16	1929.53	1826.641	
Storey 06	3052	2102.97	2000.081	
Storey 05	3280.1	2254.83	2151.941	
Storey 04	3327.98	2367.09	2264.201	
Storey 03	3579.09	2579.21	2476.321	
Storey 02	3896.87	2769.83	2666.941	
Storey 01	4079.65	2981.18	2878.291	
Base	4178.28	3199.29	3096.401	

Table 4 Axial Force in kN in column

## V. CONCLUSION

# Storey Displacement

The lateral displacement of the story in relation to the base is known as story displacement. The building's severe lateral displacement can be limited by the lateral force-resisting system. For a wind load situation, the acceptability lateral displacement limit might be H/500 (others may choose H/400). In comparison to structures with AAC block infill and structures with interlocking Block Infill, the largest Storey Displacement was found at the top storey for structures with masonry infill.

## Storey Drift

In general, one would subtract the narrative displacement of level "X" from the story displacement

of level "X-1" to determine the story drift of level "X." For example, level 4's storey drift is equal to level 4's total story displacement minus level 3's story displacement. The storey drift in any storey due to the minimum required design lateral force, with a partial load factor of 1.0, shall not exceed 0.004 times the storey height, according to IS 1893 (part 1): 2002 CI. 7.11. 3. In comparison to structures with AAC Block and structures with Interlocking Block, storey drift was greatest at Storey 8 in structures with masonry infill.

## Storey Shear

The ratio of the story shear force when story collapse occurs to the story shear force when entire collapse happens is known as the Storey shear factor. Simple formulae are proposed as a result of a series of



dynamic analyses to compute the necessary story shear safety factor that may be employed to prevent story collapse. In comparison to other situations, storey shear was 28 percent higher in structures with masonry infill. The results showed that structures made up of interlocking blocks outperformed those made up of AAC blocks.

# Axial Force

Axial load is determined by the type of reinforcement utilised in the column as well as the column's size. Axial load is a structural load that acts on the longitudinal axis of a column and is created by a beam slab and a brick wall. Axial loading of a column means that the load is operating on the column's longitudinal axis, which causes no moment. The following is a general formula for axial force: Where E is 7,000 times MPA, KN equals 1,000 times Newton, and d equals 640.3 times mm, Ned equals 270 times KN. Assume that due to centrifugal force, the segments furthest from the door interception are positive and those closest to the door are negative. When compared to the other two cases, the axial load in the structure with masonry infill was 8% higher. **Bending Moment** 

The coefficients of moments can be calculated by dividing the support moments by the span length. Because the support moment is shared by two spans on the left and right, the coefficient of moment is determined by averaging these spans. Apart from being 6.8% higher in structure with masonry infill when compared to structure with AAC Block infill and 7.1 percent higher when compared to structure with interlocking Bricks infill in all three cases, bending moment was found to be 6.8% higher in structure with masonry infill when compared to structure with AAC Block infill and 7.1 percent higher when compared to structure with interlocking Bricks infill in all three cases.

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