

Dynamic Analysis of a Circular Tall Structure Considering Outriggers using ETABS

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| Article Info | ABSTRACT |
|---------------------------|---|
| March-April-2022 | Deeply and outrigger underlying frameworks. Deeply and less powers are |
| | conveyed by the structure's fringe. This approach was likewise utilized for a |
| Publication Issue : | structure of medium stature, notwithstanding tall designs. |
| Volume 6, Issue 2 | The objective of this study is to take a gander at the seismic variety in round |
| Page Number : 168-175 | structures by putting the Outrigger framework at the inward divider in the built |
| | up substantial design. |
| Article History | In this review, there are two models: one is a customary roundabout structure, |
| Accepted : 01 April 2022 | and the other is a structure with an outrigger framework incorporated into the |
| Published : 09 April 2022 | inward divider. Dynamic examination and rotational seismic investigation are |
| | the techniques used to break down these models, and Etabs is the product used |
| | to do as such. I.S. (Indian Standard) 1893 section 1: 2016 was the code utilized |
| | for this unique review. In the wake of surveying all models, the models are |
| | looked at on the seismic boundary values (parallel story force, story dislodging, |
| | periods and recurrence, story solidness) so that each model might be able to see |
| | which model is the most steady. |
| | Keywords : Core and Outrigger structural system, Connection design, Dynamic |

analysis, time history, Rotational Seismic Analysis and Etabs.

I. INTRODUCTION

These days, each RCC building is intended to be quake safe on the grounds that the stature of the structure is expanding step by step, expanding the upsetting second, making the structure fall flat in the toppling. In the event that the stature of the structure is low, the worth of the upsetting second and base shear is low, on the grounds that the worth of the base shear and toppling second is subject to the design's self-weight. Outrigger framework is given at a different area in this round shape development to decrease the impact of dynamic (wind and seismic) on the construction. The material used in round structures is 15 to 18% not exactly in rectangular structures. Making an association between a roundabout segment and a rectangular bar is extremely basic in a roundabout molded building. Since Courtyards round building gave open space inside it, all models of this paper fall under its umbrella.

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Seismic Rotational Loading

The excitation of designs brought about by the torsional and shaking parts of seismic exercises is alluded to as tremor rotational stacking. Nathan M. Newmark was quick to show that this sort of stacking could cause unexpected primary disappointment, and that its effect ought to be figured into plan rules. The engendering of body waves, surface waves, explicit rotational waves, block revolution, geology impact, and soil structure collaboration are generally potential reasons for tremor rotational stacking of designs.

Characterizing trustworthy and exact stacking designs for the plan of tremor safe constructions in light of all parts of seismic movements, for example three translational and three rotational parts, is one of the issues in underlying designing. From the outlook of tremor designing, it is regularly viewed as that the rotational parts of critical ground movements are brought about by the spatial variety of seismic waves, and that these parts are accordingly assessed regarding comparable translational parts. While quake shaking can be depicted at a solitary spot, point pivot, which compares to the angle of a point on the ground surface, can be utilized to perform rotational stacking of constructions. Most investigations of seismic tremor rotational stacking have observed that the rotational parts, in light of their recurrence content, can fundamentally adjust the unique way of behaving of designs that are touchy to high-recurrence movements, for example, auxiliary frameworks, recorded landmarks, atomic reactors, tall hilter kilter structures or unpredictable edges, thin pinnacle shape constructions, spans, and in an upward direction irrascible designs. Whenever the effects of kinematic and dynamic soil structure communication are considered in underlying stacking and demonstrating, the commitment of rotational parts to the seismic reaction of designs upheld on unbending mat establishments might actually be upgraded.

Outrigger Structural System

Outriggers are horizontal underlying frameworks introduced inside skyscraper designs to improve upsetting solidness and strength. It's a structure based parallel burden opposing framework. The whole framework is comprised of outriggers, which are underlying parts that interface the center design to the structure's external segments. Level pillars, bracket, and dividers can be utilized as outriggers.



Fig 1. Outrigger Structural System

Outriggers are interior structural systems that can withstand the weight of up to 150 floors. It is one of the most successful and steady high-rise building configurations. Because of its unique mix of architectural freedom and structural efficiency, the outrigger structural system has been popular in building since the 1980s.

II. LITERATURE REVIEW

Gaurav Patidar and Ankur Pandey (2022) The seismic and wind investigation of multi-story structures with different unpredictable and muddled plan shapes was portrayed in this study work, which produced into account the results of shear dividers, seismic zone variety, and wind speed. Story float, parallel dislodging, base shear, story shear, delicate story, hub



power, minutes, and different impacts of plan shape were investigated.

As per the discoveries, customary structures have a higher base shear, suggesting more solidness. As the structure's story tallness rises, the firmness diminishes. The highest level has the most story float, though the moderate story has the least. Under wind loads, unpredictable structures have more relocation than normal structures, while under seismic burdens, sporadic structures have less dislodging than customary structures.

Krishna Prasad Chaudhary and Ankit Mahajan (2021) The investigation of tall structures using CSI ETABS under the effect of the reaction range was accounted for in this examination work. A few particular framed elevated structure structures, for example, H molded, O formed, and C molded structures, were thought of, with every one of the three molded structures having an alternate level, going from 12 to 16 stories.

Twisting minutes and shear powers were expanded from 1.17 percent to 1.84 percent, as per the discoveries. The O-formed building has the most variety in B.M. what's more, S.F. Every one of the three sporadic shapes, Hshape, L-molded, and Oformed, cause the most dislodging when worked as a L-formed building.

Basavaraju S N and Kavitha S (2020) The review's fundamental objective was to find the best and weak development shape in calamity inclined regions. The impacts of six particular arrangement designs with a similar arrangement region (144 m2) were contemplated: Rectangular, Circular. Square, Triangular, Pentagon, and Hexagon. As opposed to quake and wind stresses, mono section structures with contrasting arrangement calculation respond adversely. CSI ETABS programming was utilized to display, break down, and plan the Multi story mono section development. Most extreme upsides of Story removal, Story float, Story shear, upsetting second, Story firmness, and Time period were determined utilizing seismic and wind examinations.

The outcomes showed that in every seismic zone, story dislodging and story float values are at their most minimal in roundabout structure structures. In a square structure, story shear, upsetting second, and story solidness are negligible, however in a triangle shape structure, they are maximal. A mono segment development with a rectangular arrangement setup has the most significant length of time term. Because of extensive dislodging, mono section constructions of the Rectangular shape are not suggested in high seismic and high wind inclined zones. Mono segment structures are not proper for unpredictable shapes. The exhibition of a mono section structure with a roundabout, three-sided, or hexagonal shape plan course of action against seismic and wind loads is amazing.

III. CONCLUSION

In this study we observed that authors in past analysed structures considering various analysis method to determine the effect over the structure but none of them analysed a cirvular tall structure for seismic load.

Objectives of the research

- To create, arranging and investigation model of the roundabout design G+10 in E-Tabs programming.
- To evaluate rotational seismic exhibitions of roundabout design with outrigger framework at internal divider and without outrigger framework .
- To Determine the greatest story uprooting, story shear, story float, toppling second, story solidness and time of roundabout



data

construction for round setup subject to seismic burden.

• To decide the usage of etabs programming in examination of a roundabout tall designs.

IV. Methodology

Step 1: Numerous research papers were reviewed from authors all around the globe who have particularly worked on research based on rotational seismic analysis to define structure behaviour in such condition.

Step 2: The primary step is model initialization so as to define the codes and units for measurement. Here the display units are defined as per Metric SI and steel design code and concrete design code is defined as per IS 800:2007 and IS 456:2000.

| Model Initialization | | × |
|-----------------------------------|-------------|--------|
| Initialization Options | | |
| O Use Saved User Default Settings | | 0 |
| O Use Settings from a Model File | | 0 |
| Use Built-in Settings With: | | |
| Display Units | Metric SI | ~ () |
| Steel Section Database | Indian | \sim |
| Steel Design Code | IS 800:2007 | ~ () |
| Concrete Design Code | IS 456:2000 | ~ () |
| | | |
| ОК | Cancel | |
| | | |

Fig 2 Model Initialization

Step 3: Defining storey data as in this case, G+10 storey structure is considered with storey height of 3m and total elevation of 27m.

| Story10 Story9 Story8 Story7 Story6 Story5 Story4 | 3 3 3 3 3 3 3 | 27 24 21 18 15 | No No No | None None None None | No No | 0 0 0 |
|---|---|--|---|--|--|---|
| Story9 Story8 Story7 Story6 Story5 Story4 | 3 3 3 3 3 3 | 24 21 18 15 | No No No | None None None | No No | 0 |
| Story8 Story7 Story6 Story5 Story4 | 3 3 3 3 | 21 18 15 | No | None None | No | 0 |
| Story7 Story6 Story5 Story4 | 3 3 3 | 18 15 | No | None | | |
| Story6 Story5 Story4 | 3 | 15 | | | INO | 0 |
| Story5 Story4 | 3 | | No | None | No | 0 |
| Story4 | | 12 | No | None | No | 0 |
| | 3 | 9 | No | None | No | 0 |
| Story3 | 3 | 6 | No | None | No | 0 |
| Story2 | 3 | 3 | No | None | No | 0 |
| Story1 | 3 | 0 | No | None | No | 0 |
| Base | | -3 | | | | |
| ick on Grid for Optic lerging and Projecti es | ng 0.25 | m | | | | |
| | 1 | | | 01 | C 1 | |
| | Story2 Story1 Base ick on Grid for Optic lenging and Projecti es | Story2 3 Story1 3 Base | Story 2 3 3 Story 1 3 0 Base -3 3 ek on Gkt for Options | Story2 3 3 No Story1 3 0 No Base -3 ck on Gad for Options erging and Projecting | Story2 3 3 No None Story1 3 0 No None Base -3 -3 -3 -3 | Story 2 3 3 No None No Story 1 3 0 No None No Base -3 -3 -3 -3 -3 |

Fig 3 Storey Data

Step 3: The modelling of the structure using storey



Fig 4 2D and 3D view of the model

Step 4 Defining the material and section properties for the case study



Fig 5 Defining wall stack



| | | Load Cases |
|--|---|--|
| General Data | | Load Case Name |
| Property Name | SLAB SECTION | Dead |
| Slab Material | STD-CONCRETE ~ | Live |
| Notional Size Data | Modify/Show Notional Size | EQ |
| Modeling Type | Membrane 🗸 | |
| Modifiers (Currently Default) | Modify/Show | |
| Display Color | Change | |
| Property Notes | Modify/Show | |
| ✓ Use Special One-Way Load D | Distribution | |
| Property Data | | |
| Туре | Slab ~ | Fig 8 Defining |
| Thickness | 200 mm | |
| | | Full Load Displacement Cont |
| | | Quasi-Static (run a |
| OK | Cancel | Control Displacement |
| | Callee | |
| | Californi | O Use Conjugate Dis |
| | | Use Conjugate Dis |
| Fig 6 Defini | ng properties of Slab | Use Conjugate Disp Use Monitored Disp Load to a Monitored D |
| Fig 6 Defini | ng properties of Slab | Use Conjugate Dis; Use Monitored Disp Load to a Monitored D Monitored Displacement |
| Fig 6 Defini /all Property Data General Data | ng properties of Slab | Use Conjugate Disp Use Monitored Disp Load to a Monitored D Monitored Displacement © DOF/Joint |
| Fig 6 Defini /all Property Data General Data Property Name | | Use Conjugate Disp Use Monitored Disp Load to a Monitored D Monitored Displacement DOF/Joint Generalized Displa |
| Fig 6 Defini /all Property Data General Data Property Name Property Type | Ing properties of Slab | Use Conjugate Disp Use Monitored Disp Load to a Monitored D Monitored Displacement ODF/Joint Generalized Displa |
| Fig 6 Defini 'all Property Data General Data Property Name Property Type Wall Material | Ing properties of Slab | Use Conjugate Disp Use Monitored Disp Load to a Monitored Di Monitored Displacement ODF/Joint Generalized Displa Quasi-static Parameters Time History Type |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data | ng properties of Slab | Use Conjugate Disp Use Monitored Disp Load to a Monitored Di Monitored Displacement © DOF/Joint Generalized Displa Quasi-static Parameters Time History Type Output Time Step Size |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type | ng properties of Slab | Use Conjugate Disp Use Monitored Disp Load to a Monitored Displacement ODF/Joint Generalized Displa Quasi-static Parameters Time History Type Output Time Step Size Mass Proportional Dan |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type Modifiers (Currently Default) | DUTRIGGER Specified SD-CONCRETE Modify/Show Notional Size | Use Conjugate Disp Use Monitored Disp Load to a Monitored Disp Monitored Displacement DOF/Joint Quasi-static Parameters Time History Type Output Time Step Size Mass Proportional Dan Hilber-Hughes-Taylor |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type Modifiers (Currently Default) Display Color | Ing properties of Slab | Use Conjugate Disp Use Monitored Disp Load to a Monitored Displacement ODF/Joint Generalized Displact Quasi-static Parameters Time History Type Output Time Step Size Mass Proportional Dan Hilber-Hughes-Taylor T |
| Fig 6 Defini all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type Modfiers (Currently Default) Display Color Property Notes | Ing properties of Slab | Use Conjugate Displacement Use Monitored Displacement Monitored Displacement DOF/Joint Generalized Displa Quasi-static Parameters- Time History Type Output Time Step Size Mass Proportional Dan Hilber-Hughes-Taylor 1 |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type Modifiers (Currently Default) Display Color Property Notes | DUTRIGGER Specified SD-CONCRETE Modfy/Show Modfy/Show Modfy/Show | Use Conjugate Disp Use Monitored Displacement Monitored Displacement DOF/Joint Generalized Displa Quasi-static Parameters- Time History Type Output Time Step Size Mass Proportional Dan Hilber-Hughes-Taylor T |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type Modifiers (Currently Default) Display Color Property Notes Property Data | DUTRIGGER Specified STD-CONCRETE Modfy/Show Shell-Thin Modfy/Show Change Modfy/Show | Use Conjugate Disp ● Use Monitored Disp Load to a Monitored Displacement ● DOF/Joint ● Generalized Displa Quasi-static Parameters Time History Type Output Time Step Size Mass Proportional Dan Hilber-Hughes-Taylor |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type Modifiers (Currently Default) Display Color Property Data Property Data Thickness | Ing properties of Slab | ○ Use Conjugate Disj ○ Use Monitored Displacement ○ ODF/Joint ○ Generalized Displa ○ Quasi-static Parameters- Time History Type ○ Output Time Step Size Mass Proportional Dan Hilber-Hughes-Taylor 1 Fig 9 Load app |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type Modifiers (Currently Default) Display Color Property Notes Property Data Thickness | Ing properties of Slab MUTRIGGER Specified SD-CONCRETE Modify/Show Change Modify/Show 200 mm | Use Conjugate Disp Use Monitored Disp Load to a Monitored Displacement DOF/Joint Quasi-static Parameters Time History Type Output Time Step Size Mass Proportional Dan Hilber-Hughes-Taylor T Fig 9 Load app |
| Fig 6 Defini /all Property Data General Data Property Type Wall Material Notional Size Data Modeling Type Modifiers (Currently Default) Display Color Property Data Thickness | Ang properties of Slab | Use Conjugate Displacement ● Use Monitored Displacement ● DOF/Joint ● Generalized Displacement ● Quasi-static Parameters Time History Type Output Time Step Size Mass Proportional Darn Hilber-Hughes-Taylor |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type Modifiers (Currently Default) Display Color Property Data Thickness OK | Ang properties of Slab | Use Conjugate Displacement ● Use Monitored Displacement ● DDF/Joint ● Generalized Displacement ● Gen |
| Fig 6 Defini /all Property Data General Data Property Name Property Type Wall Material Notional Size Data Modeling Type Modifiers (Currently Default) Display Color Property Data Thickness OK Fig 7 Defining pr | Ing properties of Slab | Use Conjugate Disp Use Monitored Disp Load to a Monitored Disp Load to a Monitored Displacement DOF/Joint Cuasi-static Parameters Time History Type Output Time Step Size Mass Proportional Dan Hilber-Hughes-Taylor T Fig 9 Load app Solution Control Maximum Total Steps Use Ferration Maximum Total Steps Use Terration Maximum Total Steps Maximum Total Steps Maxim |



| Eull Load | itrol | | | | | | |
|--------------------|-----------------|--------------|------------|-------------|-------------|-----------------|-------|
| Displacement (| Control | | | | | | |
| | Control | | | | | | |
| Quasi-Static (r | run as time his | tory) | | | | | |
| ntrol Displacement | t | | | | | | |
| Use Conjugate | Displacement | | | | | | |
| Use Monitored | Displacement | | | | | | |
| Load to a Monitor | ed Displaceme | nt Magnitude | e of | | | | _ |
| | | | | | | | |
| nitored Displacem | ent | | | | | | _ |
| DOF/Joint | 01 | ~ | Story10 | | | × 1 | _ |
| Generalized D | isplacement | | | | | | |
| asi-static Paramet | ters | | | | | | |
| Time History Type | | | | Nonlinear [| Direct Inte | gration History | |
| Output Time Step | Size | | | , | | 1 | sec |
| Mass Proportiona | I Damping | | | | | 0 | 1/sec |
| Hilber-Hughes-Ta | ylor Time Integ | ration Param | neter, Alp | ha | | 0 | |
| | | | 1 | | | - | |

Fig 9 Load application control for Non linear static analysis

| 1 | Solution Control | | |
|----|---|--------|--|
| | Maximum Total Steps | 200 | |
| | Maximum Null Steps | 50 | |
| | Use Event-To-Event Stepping | Yes | |
| | Event Lumping Tolerance (Relative) | 0.01 | |
| | Use Iteration | Yes | |
| | Maximum Constant-Stiffness Iterations | 10 | |
| | Maximum Newton-Raphson Iterations | 40 | |
| | Iteration Convergence Tolerance (Relative) | 0.0001 | |
| | Use Line Search | Yes | |
| | Maximum Line Searches per Iteration | 20 | |
| | Line Search Acceptance Tolerance (Relative) | 0.1 | |
| | Line Search Step Factor | 1.618 | |
| | Material Nonlinearity Parameters | | |
| 4a | Material Nonlinearity Parameters | | |



Fig 10 Parameters for Non Linear Analysis

| Direction and Eccentricity | | Seismic Coefficients | | | |
|--|--|--|-----------------|-------------------|-----|
| X Dir X Dir + Eccentricity X Dir - Eccentricity Ecc. Ratio (All Diaph.) Overwrite Eccentricities | Y Dir Y Dir + Eccentricity Y Dir - Eccentricity 0.05 Overwrite | Seismic Zone Factor, Z Per Code User Defined Site Type Importance Factor, I | | 0.36 III 12 | ~ |
| Story Range Top Story Bottom Story Factors | Story10 v Base v | Time Period Approximate Program Calculated User Defined | Ct (m) = T = | | sec |

Fig 11 Seismic Load Pattern as per IS 1893:2002.

Step 6: Analyzing results on the parameters of storey displacement, shear force, bending moment.



(a)



Fig 13 Analytical Results Table 1: Geomerical description:

| Beam | 250 mmX400 mm |
|---------------------|-----------------|
| Column | 400 mm diameter |
| Slab | 200 mm |
| Span of Beam | 4.0 m |
| Height of building | 27 m |
| Floor height | 3.0m |
| Ground storey | 3.0m |
| External Diameter | 72.0m |
| Internal Diameter | 32.0m |
| Area | 430 m2 |
| Outrigger Thickness | 200 mm |

Analysis result:

Circular Structure/X-axis, Circular Structure/Y-axis, Circular Structure with Outrigger System/X-axis and Circular Structur...



| Base Shear in kN | | | | | | | |
|------------------|---------------|----------------|--------------------------------|--|--|--|--|
| Circul | lar Structure | Circular Struc | eture with Outrigger System | | | | |
| X-axis | Y-axis | X-axis | Y-axis | | | | |
| 3080.596 | 3049.694 | 1835.931 | 1928.773 | | | | |



Circular Structure/X-axis, Circular Structure/Y-axis, Circular Structure with Outrigger System/X-axis and Circular Structur...



Circular Structure and Circular Structure with Outrigger System



V. CONCLUSION

Story Displacement

Conversation: The parallel relocation of the story according to the premise is known as story uprooting. The structure's exorbitant horizontal dislodging can be restricted by the parallel power opposing system.Storey relocation was tracked down greatest in roundabout design though the round structure with outrigger framework had the option to hold the uprooting and acted stable.

Base Shear

Conversation: The most extreme expected sidelong weight on the foundation of the design inferable from seismic movement is called base shear. It's assessed utilizing the seismic zone, soil material, and parallel power formulae from the building regulation. Here for this situation base shear was in excess of 3000 kN in both X and Y course in Case I and under 2000 kN in X and Y bearing for Case II.

Deflection

Conversation: In designing, diversion is how much a piece of an underlying component is uprooted under a heap (since it twists). It might allude to a point or a distance. Diversion was 4% more in Case I when contrasted with case II.

Story Drift

Story float is the parallel dislodging of a story comparative with the floor beneath, and the story float proportion is the story float isolated by the story tallness. The story float was evaluted for X and Y course in both the cases with a variety of 3% in the outcomes.

Time Period

According to time history strategy it has been seen that the upsides of base shear and popular narrative relocation are higher in Case I in contrast with Case II. Regular time-frame was higher on the off chance that I thus, the design with outrigger framework was seen as more reasonable to diminish the time span.

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Cite this article as :

Brijdeep Singh Bhatia, Pradeep Kumar Nirmal, Lokesh Singh, "Dynamic Analysis of a Circular Tall Structure Considering Outriggers using ETABS", International Journal of Scientific Research in Civil Engineering (IJSRCE), ISSN : 2456-6667, Volume 6 Issue 2, pp. 168-175, March-April 2022. URL : https://ijsrce.com/IJSRCE226216